

TELJESÍTMÉNYNYILATKOZAT  
DoP Nr. MKT-111 - hu

1. A terméktípus egyedi azonosító kódja: **MKT Bolzenanker BZ plus & BZ-IG**
2. Típus-, tétel- vagy sorozatszám vagy egyéb ilyen elem, amely lehetővé teszi az építési termék azonosítását a 11. cikk (4) bekezdésében előírtaknak megfelelően:

**ETA-99/0010, Melléklet A3, A5**  
**Gyártási szám a csomagoláson vagy a címkén**

3. Az építési terméknek a gyártó által meghatározott rendeltetése vagy rendeltetései az alkalmazandó harmonizált műszaki előírással összhangban:

<b>általános típus</b>	Nyomaték kontrollált feszítő tőcsavar (ék típusú)
<b>való használatra</b>	Repedéses és repedésmentes betonban C20/25 - C50/60 (EN 206)
<b>opció</b>	1
<b>terhelés</b>	Statikus vagy kvázi-statikus, szeizmikus, kategória C1+C2 (Méretek BZ plus M10, M12, M16, M20)
<b>anyag</b>	<u>Horganyzott acél:</u> száraz belső használatra Méretek: BZ plus: M8, M10, M12, M16, M20, M24, M27 BZ-IG: M6, M8, M10, M12 <u>Rozsdamentes acél (jelölés A4):</u> belső és külső használatra nem különösen agresszív feltételek Méretek: BZ plus: M8, M10, M12, M16, M20, M24 BZ-IG: M6, M8, M10, M12 <u>Magas korrózióállóságú acél (jelölés HCR):</u> beltéri és kültéri alkalmazás különösen agresszív körülmények között Méretek: BZ plus: M8, M10, M12, M16, M20, M24 BZ-IG: M6, M8, M10, M12
<b>hőmérséklet tartomány, ha alkalmazható</b>	--

4. A gyártók neve, bejegyzett kereskedelmi neve, illetve bejegyzett védjegye, valamint értesítési címe a 11. cikk (5) bekezdésében előírtaknak megfelelően:

**MKT Metall-Kunststoff-Technik GmbH & Co. KG**  
**Auf dem Immel 2**  
**D - 67685 Weilerbach**

5. Adott esetben annak a meghatalmazott képviselőnek a neve és értesítési címe, akinek a megbízása körébe a 12. cikk (2) bekezdésében meghatározott feladatok tartoznak: --
6. Az építési termékek teljesítménye állandóságának értékelésére és ellenőrzésére szolgáló, az V. mellékletben szereplők szerinti rendszer vagy rendszerek: **rendszer 1**
7. Harmonizált szabványok által szabályozott építési termékekre vonatkozó gyártói nyilatkozat esetén: --

8. Olyan építési termékekre vonatkozó gyártói nyilatkozat esetén, amelyekre európai műszaki értékelést adtak ki:

**Deutsches Institut für Bautechnik, Berlin**

a következőt adta ki

**ETA-99/0010**

alapján

**ETAG 001-2**

A bejelentett termék tanúsító szervezet 1343-CPR tette, hogy a rendszer 1:

- i) a terméktípus meghatározása típusvizsgálat (ideértve a mintavételt is), típusszámítás, táblázatba foglalt értékek vagy a termék leíró dokumentációja alapján;
- ii) a gyártó üzem és az üzemi gyártásellenőrzés alapvizsgálata;
- iii) az üzemi gyártásellenőrzés folyamatos felügyelete, vizsgálata és értékelése.

és a következőt adta ki: a termék megfelelőségi tanúsítványa 1343-CPR-M 550-1

9. A nyilatkozat szerinti teljesítmény:

Alapvető tulajdonságok	Tervezési módszer	Teljesítmény		Harmonizált műszaki előírások
		BZ plus	BZ-IG	
karakterisztikus ellenállás húzásra	ETAG 001, Melléklet C CEN/TS 1992-4	Melléklet C1-C4	Melléklet C10-C11	ETAG 001
karakterisztikus ellenállás nyírásra	ETAG 001, Melléklet C CEN/TS 1992-4	Melléklet C5	Melléklet C12	
karakterisztikus ellenállás szeizmikus kereslet	TR 045	Melléklet C6	NPD	
elmozdulás használhatósági határállapot	ETAG 001, Melléklet C CEN/TS 1992-4	Melléklet C8-C9	Melléklet C14	
karakterisztikus ellenállás tűz expozíció	TR 020 CEN/TS 1992-4	Melléklet C7	Melléklet C13	

Amennyiben a 37. és 38. cikknek megfelelően egyedi műszaki dokumentáció alkalmazására került sor, a termék által teljesített követelmények: --

10. Az 1. és 2. pontban meghatározott termék teljesítménye megfelel a 9. pontban feltüntetett, nyilatkozat szerinti teljesítménynek.

E teljesítménynyilatkozat kiadásáért kizárólag a 4. pontban meghatározott gyártó a felelős.

A gyártó nevében és részéről aláíró személy:

  
**Lore Weustenhagen**  
(menedzser)  
**Weilerbach, 09.01.2015**

i.V.   
**Dipl.-Ing. Detlef Bigalke**  
(menedzser termékfejlesztés)



**Table C1:** Characteristic values for **tension loads, BZ plus zinc plated, cracked concrete**, static and quasi-static action, design method A according to ETAG 001, Annex C or CEN/TS 1992-4

Anchor size			M8	M10	M12	M16	M20	M24	M27
Installation safety factor	$\gamma_2 = \gamma_{inst}$	[-]	1,0						
<b>Steel failure</b>									
Characteristic tension resistance	$N_{Rk,s}$	[kN]	16	27	40	60	86	126	196
Partial safety factor	$\gamma_{Ms}$	[-]	1,53		1,5		1,6	1,5	
<b>Pull-out</b>									
<b>Standard anchorage depth</b>									
Characteristic resistance in concrete C20/25	$N_{Rk,p}$	[kN]	5	9	16	25	1)	1)	1)
<b>Reduced anchorage depth</b>									
Characteristic resistance in concrete C20/25	$N_{Rk,p,red}$	[kN]	5	7,5	1)	1)			
Increasing factor for $N_{Rk,p}$ and $N_{Rk,p,red}$	$\psi_c$	[-]	$\left(\frac{f_{ck,cube}}{25}\right)^{0,5}$						
<b>Concrete cone failure</b>									
Effective anchorage depth	$h_{ef}$	[mm]	46	60	70	85	100	115	125
Reduced anchorage depth	$h_{ef,red}$	[mm]	35 <sup>2)</sup>	40	50	65			
Factor for cracked concrete	$k_{cr}$	[-]	7,2						

1) Pull-out is not decisive.

2) Use restricted to anchoring of structural components statically indeterminate.

**Wedge Anchor BZ plus**

**Performance**

Characteristic values for **tension loads, BZ plus zinc plated cracked concrete**, static and quasi-static action, design method A according to ETAG 001, Annex C or CEN/TS 1992-4

**Annex C1**

**Table C2:** Characteristic values for **tension loads**, BZ plus **A4 / HCR**, **cracked concrete**, static and quasi-static action, design method A according to ETAG 001, Annex C or CEN/TS 1992-4

Anchor size			M8	M10	M12	M16	M20	M24
Installation safety factor	$\gamma_2 = \gamma_{inst}$	[-]	1,0					
<b>Steel failure</b>								
Characteristic tension resistance	$N_{Rk,s}$	[kN]	16	27	40	64	108	110
Partial safety factor	$\gamma_{Ms}$	[-]	1,5				1,68	1,5
<b>Pull-out</b>								
<b>Standard anchorage depth</b>								
Characteristic resistance in concrete C20/25	$N_{Rk,p}$	[kN]	5	9	16	25	1)	40
<b>Reduced anchorage depth</b>								
Characteristic resistance in concrete C20/25	$N_{Rk,p,red}$	[kN]	5	7,5	1)	1)		
Increasing factor for $N_{Rk,p}$ and $N_{Rk,p,red}$	$\psi_c$	[-]	$\left(\frac{f_{ck,cube}}{25}\right)^{0,5}$					
<b>Concrete cone failure</b>								
Effective anchorage depth	$h_{ef}$	[mm]	46	60	70	85	100	125
Reduced anchorage depth	$h_{ef,red}$	[mm]	35 <sup>2)</sup>	40	50	65		
Factor for cracked concrete	$k_{cr}$	[-]	7,2					

1) Pull-out is not decisive.

2) Use restricted to anchoring of structural components statically indeterminate.

**Wedge Anchor BZ plus**

**Performance**

Characteristic values for **tension loads**, BZ plus **A4 / HCR**, **cracked concrete**, static and quasi-static action, design method A according to ETAG 001, Annex C or CEN/TS 1992-4

**Annex C2**

**Table C3: Characteristic values for tension loads, BZ plus zinc plated, non-cracked concrete, static and quasi-static action, design method A according to ETAG 001, Annex C or CEN/TS 1992-4**

Anchor size			M8	M10	M12	M16	M20	M24	M27
Installation safety factor	$\gamma_2 = \gamma_{inst}$	[-]	1,0						
<b>Steel failure</b>									
Characteristic tension resistance	$N_{Rk,s}$	[kN]	16	27	40	60	86	126	196
Partial safety factor	$\gamma_{Ms}$	[-]	1,53		1,5		1,6	1,5	
<b>Pull-out</b>									
<b>Standard anchorage depth</b>									
Characteristic resistance in non-cracked concrete C20/25	$N_{Rk,p}$	[kN]	12	16	25	35	1)	1)	1)
<b>Reduced anchorage depth</b>									
Characteristic resistance in non-cracked concrete C20/25	$N_{Rk,p,red}$	[kN]	7,5	9	1)	1)			
<b>Splitting</b> For the proof against splitting failure $N^0_{Rk,c}$ has to be replaced by $N^0_{Rk,sp}$ with consideration of the member thickness									
<b>Standard anchorage depth</b>									
Splitting for <b>standard thickness of concrete member</b> (The higher resistance of case 1 and case 2 may be applied; the values $s_{cr,sp}$ and $c_{cr,sp}$ may be linearly interpolated for the member thickness $h_{min} < h < h_{std}$ (Case 2); $\psi_{h,sp} = 1,0$ )									
Standard thickness of concrete	$h_{min,1} \geq$	[mm]	100	120	140	170	200	230	250
<b>Case 1</b>									
Characteristic resistance in non-cracked concrete C20/25	$N^0_{Rk,sp}$	[kN]	9	12	20	30	40	1)	50
Spacing (edge distance)	$s_{cr,sp} (= 2 c_{cr,sp})$	[mm]	3 $h_{ef}$						
<b>Case 2</b>									
Characteristic resistance in non-cracked concrete C20/25	$N^0_{Rk,sp}$	[kN]	12	16	25	35	1)	1)	1)
Spacing (edge distance)	$s_{cr,sp} (= 2 c_{cr,sp})$	[mm]	4 $h_{ef}$				4,4 $h_{ef}$	3 $h_{ef}$	5 $h_{ef}$
<b>Splitting for minimum thickness of concrete member</b>									
Minimum thickness of concrete	$h_{min,2} \geq$	[mm]	80	100	120	140			
Characteristic resistance in non-cracked concrete C20/25	$N^0_{Rk,sp}$	[kN]	12	16	25	35			
Spacing (edge distance)	$s_{cr,sp} (= 2 c_{cr,sp})$	[mm]	5 $h_{ef}$						
<b>Reduced anchorage depth</b>									
Minimum thickness of concrete	$h_{min,3} \geq$	[mm]	80	80	100	140			
Characteristic resistance in non-cracked concrete C20/25	$N^0_{Rk,sp}$	[kN]	7,5	9	1)	1)			
Spacing (edge distance)	$s_{cr,sp} (= 2 c_{cr,sp})$	[mm]	200	200	250	300			
Increasing factor for $N_{Rk,p(red)}$ and $N^0_{Rk,sp}$	$\psi_c$	[-]	$\left(\frac{f_{ck,cube}}{25}\right)^{0,5}$						
<b>Concrete cone failure</b>									
Effective anchorage depth	$h_{ef}$	[mm]	46	60	70	85	100	115	125
Reduced anchorage depth	$h_{ef,red}$	[mm]	35 <sup>2)</sup>	40	50	65			
Factor for non-cracked concrete	$k_{Ucr}$	[-]	10,1						

<sup>1)</sup> Pull-out is not decisive.

<sup>2)</sup> Use restricted to anchoring of structural components statically indeterminate.

### Wedge Anchor BZ plus

#### Performance

Characteristic values for **tension loads, BZ plus zinc plated, non-cracked concrete**, static and quasi-static action, design method A according to ETAG 001, Annex C or CEN/TS 1992-4

**Annex C3**

**Table C4: Characteristic values for tension loads, BZ plus A4 / HCR, non-cracked concrete, static and quasi-static action, design method A according to ETAG 001, Annex C or CEN/TS 1992-4**

Anchor size			M8	M10	M12	M16	M20	M24
Installation safety factor	$\gamma_2 = \gamma_{inst}$	[-]	1,0					
<b>Steel failure</b>								
Characteristic tension resistance	$N_{Rk,s}$	[kN]	16	27	40	64	108	110
Partial safety factor	$\gamma_{Ms}$	[-]	1,5				1,68	1,5
<b>Pull-out</b>								
<b>Standard anchorage depth</b>								
Characteristic resistance in non-cracked concrete C20/25	$N_{Rk,p}$	[kN]	12	16	25	35	1)	1)
<b>Reduced anchorage depth</b>								
Characteristic resistance in non-cracked concrete C20/25	$N_{Rk,p,red}$	[kN]	7,5	9	1)	1)		
<b>Splitting</b> For the proof against splitting failure $N^0_{Rk,c}$ has to be replaced by $N^0_{Rk,sp}$ with consideration of the member thickness								
<b>Standard anchorage depth</b>								
Splitting for <b>standard thickness of concrete member</b> (The higher resistance of case 1 and case 2 may be applied; the values $s_{cr,sp}$ and $c_{cr,sp}$ may be linearly interpolated for the member thickness $h_{min} < h < h_{std}$ (Case 2); $\psi_{h,sp} = 1,0$ )								
Standard thickness of concrete	$h_{min,1} \geq$	[mm]	100	120	140	160	200	250
<b>Case 1</b>								
Characteristic resistance in non-cracked concrete C20/25	$N^0_{Rk,sp}$	[kN]	9	12	20	30	40	
Spacing (edge distance)	$s_{cr,sp} (= 2 c_{cr,sp})$	[mm]	3 $h_{ef}$					
<b>Case 2</b>								
Characteristic resistance in non-cracked concrete C20/25	$N^0_{Rk,sp}$	[kN]	12	16	25	35	1)	1)
Spacing (edge distance)	$s_{cr,sp} (= 2 c_{cr,sp})$	[mm]	230	250	280	400	440	500
<b>Splitting for minimum thickness of concrete member</b>								
Minimum thickness of concrete	$h_{min,2} \geq$	[mm]	80	100	120	140		
Characteristic resistance in non-cracked concrete C20/25	$N^0_{Rk,sp}$	[kN]	12	16	25	35		
Spacing (edge distance)	$s_{cr,sp} (= 2 c_{cr,sp})$	[mm]	5 $h_{ef}$					
<b>Reduced anchorage depth</b>								
Minimum thickness of concrete	$h_{min,3} \geq$	[mm]	80	80	100	140		
Characteristic resistance in non-cracked concrete C20/25	$N^0_{Rk,sp}$	[kN]	7,5	9	1)	1)		
Spacing (edge distance)	$s_{cr,sp} (= 2 c_{cr,sp})$	[mm]	200	200	250	300		
Increasing factor for $N_{Rk,p(red)}$ and $N^0_{Rk,sp}$	$\psi_c$	[-]	$\left(\frac{f_{ck,cube}}{25}\right)^{0,5}$					
<b>Concrete cone failure</b>								
Effective anchorage depth	$h_{ef}$	[mm]	46	60	70	85	100	125
Reduced anchorage depth	$h_{ef,red}$	[mm]	35 <sup>2)</sup>	40	50	65		
Factor for non-cracked concrete	$k_{ucr}$	[-]	10,1					

<sup>1)</sup> Pull-out is not decisive.

<sup>2)</sup> Use restricted to anchoring of structural components statically indeterminate.

### Wedge Anchor BZ plus

#### Performance

Characteristic values for **tension loads, BZ plus A4 / HCR, non-cracked concrete**, static and quasi-static action, design method A according to ETAG 001, Annex C or CEN/TS 1992-4

**Annex C4**

**Table C5:** Characteristic values for **shear loads**, BZ plus, **cracked and non-cracked concrete**, static or quasi static action, design method A according to ETAG 001, Annex C or CEN/TS 1992-4

Anchor size		M8	M10	M12	M16	M20	M24	M27	
Installation safety factor	$\gamma_2 = \gamma_{inst}$ [-]	1,0							
<b>Steel failure without lever arm, Steel zinc plated</b>									
Characteristic shear resistance	$V_{Rk,s}$ [kN]	12,2	20,1	30	55	69	114	169,4	
Factor for ductility	$k_2$ [-]	1,0							
Partial safety factor	$\gamma_{Ms}$ [-]	1,25			1,33		1,25	1,25	
<b>Steel failure without lever arm, Stainless steel A4, HCR</b>									
Characteristic shear resistance	$V_{Rk,s}$ [kN]	13	20	30	55	86	123,6	/	
Factor for ductility	$k_2$ [-]	1,0							
Partial safety factor	$\gamma_{Ms}$ [-]	1,25			1,4		1,25		
<b>Steel failure with lever arm, Steel zinc plated</b>									
Characteristic bending resistance	$M^0_{Rk,s}$ [Nm]	23	47	82	216	363	898	1331,5	
Partial safety factor	$\gamma_{Ms}$ [-]	1,25			1,33		1,25	1,25	
<b>Steel failure with lever arm, Stainless steel A4, HCR</b>									
Characteristic bending resistance	$M^0_{Rk,s}$ [Nm]	26	52	92	200	454	785,4	/	
Partial safety factor	$\gamma_{Ms}$ [-]	1,25			1,4		1,25		
<b>Concrete pry-out failure</b>									
k factor	$k_{(3)}$ [-]	2,4			2,8				
<b>Concrete edge failure</b>									
Effective length of anchor in shear loading with $h_{ef}$	Steel zinc plated	$l_f$ [mm]	46	60	70	85	100	115	125
	Stainless steel A4, HCR	$l_f$ [mm]	46	60	70	85	100	125	/
Effective length of anchor in shear loading with $h_{ef,red}$	Steel zinc plated	$l_{f,red}$ [mm]	35	40	50	65	/	/	
	Stainless steel A4, HCR	$l_{f,red}$ [mm]	35	40	50	65			
Outside diameter of anchor	$d_{nom}$ [mm]	8	10	12	16	20	24	27	

**Wedge Anchor BZ plus**

**Performance**

Characteristic values for **shear loads**, BZ plus, **cracked and non-cracked concrete**, static or quasi static action, design method A according to ETAG 001, Annex C or CEN/TS 1992-4

**Annex C5**

**Table C6:** Characteristic resistance for **seismic loading**, BZ plus, **standard anchorage depth**, performance category **C1** and **C2**, design according to TR045

<b>Tension loads</b>						
<b>Anchor size</b>			<b>M10</b>	<b>M12</b>	<b>M16</b>	<b>M20</b>
Installation safety factor	$\gamma_2 = \gamma_{inst}$	[-]	1,0			
<b>Steel failure, steel zinc plated</b>						
Characteristic resistance <b>C1</b>	$N_{Rk,s,seis,C1}$	[kN]	27	40	60	86
Characteristic resistance <b>C2</b>	$N_{Rk,s,seis,C2}$	[kN]	27	40	60	86
Partial safety factor	$\gamma_{Ms,seis}$	[-]	1,53	1,5		1,6
<b>Steel failure, stainless steel A4, HCR</b>						
Characteristic resistance <b>C1</b>	$N_{Rk,s,seis,C1}$	[kN]	27	40	64	108
Characteristic resistance <b>C2</b>	$N_{Rk,s,seis,C2}$	[kN]	27	40	64	108
Partial safety factor	$\gamma_{Ms,seis}$	[-]	1,5			1,68
<b>Pull-out</b>						
Characteristic resistance <b>C1</b>	$N_{Rk,p,seis,C1}$	[kN]	9	16	25	36
Characteristic resistance <b>C2</b>	$N_{Rk,p,seis,C2}$	[kN]	3,6	10,2	13,8	22,4

<b>Shear loads</b>						
<b>Steel failure without lever arm, Steel zinc plated</b>						
Characteristic resistance <b>C1</b>	$V_{Rk,s,seis,C1}$	[kN]	20	27	44	69
Characteristic resistance <b>C2</b>	$V_{Rk,s,seis,C2}$	[kN]	14	16,2	35,7	55,2
Partial safety factor	$\gamma_{Ms,seis}$	[-]	1,25			1,33
<b>Steel failure without lever arm, Stainless steel A4, HCR</b>						
Characteristic resistance <b>C1</b>	$V_{Rk,s,seis,C1}$	[kN]	20	27	44	69
Characteristic resistance <b>C2</b>	$V_{Rk,s,seis,C2}$	[kN]	14	16,2	35,7	55,2
Partial safety factor	$\gamma_{Ms,seis}$	[-]	1,25			1,4

**Wedge Anchor BZ plus**

**Performance**

Characteristic resistance for **seismic loading**, BZ plus, **standard anchorage depth**, performance category **C1** and **C2**, design according to TR045

**Annex C6**



**Table C7: Characteristic values for tension and shear load under fire exposure, BZ plus, standard anchorage depth, cracked and non-cracked concrete C20/25 to C50/60, design acc. to TR 020 or CEN/TS 1992-4, Annex D**

Anchor size		M8	M10	M12	M16	M20	M24	M27		
<b>Tension load</b>										
<b>Steel failure</b>										
<b>Steel zinc plated</b>										
Characteristic resistance	R30	$N_{Rk,s,fi}$	[kN]	1,4	2,2	3,2	6,0	9,4	13,6	17,6
	R60			1,1	1,8	2,8	5,2	8,2	11,8	15,3
	R90			0,8	1,4	2,4	4,4	6,9	10,0	13,0
	R120			0,7	1,2	2,2	4,0	6,3	9,1	11,8
<b>Stainless steel A4, HCR</b>										
Characteristic resistance	R30	$N_{Rk,s,fi}$	[kN]	3,8	6,9	11,5	21,5	33,5	48,2	
	R60			2,9	5,2	8,6	16	25,0	35,9	
	R90			2,0	3,5	5,6	10,5	16,4	23,6	
	R120			1,6	2,7	4,2	7,8	12,1	17,4	
<b>Shear load</b>										
<b>Steel failure without lever arm</b>										
<b>Steel zinc plated</b>										
Characteristic resistance	R30	$V_{Rk,s,fi}$	[kN]	1,6	2,6	3,8	7,0	11	16	20,6
	R60			1,5	2,5	3,6	6,8	11	15	19,8
	R90			1,2	2,1	3,5	6,5	10	15	19,0
	R120			1,0	2,0	3,4	6,4	10	14	18,6
<b>Stainless steel A4, HCR</b>										
Characteristic resistance	R30	$V_{Rk,s,fi}$	[kN]	3,8	6,9	11,5	21,5	33,5	48,2	
	R60			2,9	5,2	8,6	16	25,0	35,9	
	R90			2,0	3,5	5,6	10,5	16,4	23,6	
	R120			1,6	2,7	4,2	7,8	12,1	17,4	
<b>Steel failure with lever arm</b>										
<b>Steel zinc plated</b>										
Characteristic resistance	R30	$M^0_{Rk,s,fi}$	[Nm]	1,7	3,3	5,9	15	29	50	75
	R60			1,6	3,2	5,6	14	28	48	72
	R90			1,2	2,7	5,4	14	27	47	69
	R120			1,1	2,5	5,3	13	26	46	68
<b>Stainless steel A4, HCR</b>										
Characteristic resistance	R30	$M^0_{Rk,s,fi}$	[Nm]	3,8	9,0	17,9	45,5	88,8	153,5	
	R60			2,9	6,8	13,3	33,9	66,1	114,3	
	R90			2,1	4,5	8,8	22,2	43,4	75,1	
	R120			1,6	3,4	6,5	16,4	32,1	55,5	

The characteristic resistance for pull-out failure, concrete cone failure, concrete pry-out and concrete edge failure can be calculated according to TR020 / CEN/TS 1992-4. If pull-out is not decisive  $N_{Rk,p}$  in Eq. 2.4 and Eq. 2.5, TR 020 must be replaced by  $N^0_{Rk,c}$ .

**Wedge Anchor BZ plus**

**Performance**

Characteristic values for tension and shear load under fire exposure, BZ plus, standard anchorage depth, cracked and non-cracked concrete C20/25 to C50/60, design acc. to TR 020 or CEN/TS 1992-4, Annex D

**Annex C7**

**Table C8: Displacements under tension load, BZ plus**

Anchor size			M8	M10	M12	M16	M20	M24	M27
<b>Standard anchorage depth</b>									
<b>Steel zinc plated</b>									
Tension load in cracked concrete	N	[kN]	2,4	4,3	7,6	11,9	17,1	21,1	24
Displacement	$\delta_{N0}$	[mm]	0,6	1,0	0,4	1,0	0,9	0,7	0,9
	$\delta_{N\infty}$	[mm]	1,4	1,2	1,4	1,3	1,0	1,2	1,4
Tension load in non-cracked concrete	N	[kN]	5,7	7,6	11,9	16,7	23,8	29,6	34
Displacement	$\delta_{N0}$	[mm]	0,4	0,5	0,7	0,3	0,4	0,5	0,3
	$\delta_{N\infty}$	[mm]	0,8		1,4	0,8		1,4	
<b>Displacements under seismic tension loads C2</b>									
Displacements for DLS	$\delta_{N,seis,C2(DLS)}$	[mm]	/	4,1	4,9	3,6	5,1	/	/
Displacements for ULS	$\delta_{N,seis,C2(ULS)}$	[mm]		13,8	15,7	9,5	15,2		
<b>Stainless steel A4, HCR</b>									
Tension load in cracked concrete	N	[kN]	2,4	4,3	7,6	11,9	17,1	19,0	/
Displacement	$\delta_{N0}$	[mm]	0,7	1,8	0,4	0,7	0,9	0,5	
	$\delta_{N\infty}$	[mm]	1,2	1,4	1,4	1,4	1,0	1,8	
Tension load in non-cracked concrete	N	[kN]	5,8	7,6	11,9	16,7	23,8	33,5	/
Displacement	$\delta_{N0}$	[mm]	0,6	0,5	0,7	0,2	0,4	0,5	
	$\delta_{N\infty}$	[mm]	1,2	1,0	1,4	0,4	0,8	1,1	
<b>Displacements under seismic tension loads C2</b>									
Displacements for DLS	$\delta_{N,seis,C2(DLS)}$	[mm]	/	4,1	4,9	3,6	5,1	/	/
Displacements for ULS	$\delta_{N,seis,C2(ULS)}$	[mm]		13,8	15,7	9,5	15,2		
<b>Reduced anchorage depth</b>									
Tension load in cracked concrete	N	[kN]	2,4	3,6	6,1	9,0	/	/	/
Displacement	$\delta_{N0}$	[mm]	0,8	0,7	0,5	1,0			
	$\delta_{N\infty}$	[mm]	1,2	1,0	0,8	1,1			
Tension load in non-cracked concrete	N	[kN]	3,7	4,3	8,5	12,6	/	/	/
Displacement	$\delta_{N0}$	[mm]	0,1	0,2	0,2	0,2			
	$\delta_{N\infty}$	[mm]	0,7	0,7	0,7	0,7			

**Wedge Anchor BZ plus**

**Performance**  
Displacements under tension load

**Annex C8**

**Table C9: Displacements under shear load, BZ plus**

Anchor size			M8	M10	M12	M16	M20	M24	M27
<b>Standard anchorage depth</b>									
<b>Steel zinc plated</b>									
Shear load in cracked and non-cracked concrete	V	[kN]	6,9	11,4	17,1	31,4	36,8	64,9	96,8
Displacement	$\delta_{V0}$	[mm]	2,0	3,2	3,6	3,5	1,8	3,5	3,6
	$\delta_{V\infty}$	[mm]	3,0	4,7	5,5	5,3	2,7	5,3	5,4
<b>Displacements under seismic shear loads C2</b>									
Displacements for DLS	$\delta_{V,seis,C2(DLS)}$	[mm]	/	2,7	3,5	4,3	4,7	/	/
Displacements for ULS	$\delta_{V,seis,C2(ULS)}$	[mm]		5,3	9,5	9,6	10,1		
<b>Stainless steel A4, HCR</b>									
Shear load in cracked and non-cracked concrete	V	[kN]	7,3	11,4	17,1	31,4	43,8	70,6	/
Displacement	$\delta_{V0}$	[mm]	1,9	2,4	4,0	4,3	2,9	2,8	
	$\delta_{V\infty}$	[mm]	2,9	3,6	5,9	6,4	4,3	4,2	
<b>Displacements under seismic shear loads C2</b>									
Displacements for DLS	$\delta_{V,seis,C2(DLS)}$	[mm]	/	2,7	3,5	4,3	4,7	/	/
Displacements for ULS	$\delta_{V,seis,C2(ULS)}$	[mm]		5,3	9,5	9,6	10,1		
<b>Reduced anchorage depth</b>									
<b>Steel zinc plated</b>									
Shear load in cracked and non-cracked concrete	V	[kN]	6,9	11,4	17,1	31,4	/	/	/
Displacement	$\delta_{V0}$	[mm]	2,0	3,2	3,6	3,5			
	$\delta_{V\infty}$	[mm]	3,0	4,7	5,5	5,3			
<b>Stainless steel A4, HCR</b>									
Shear load in cracked and non-cracked concrete	V	[kN]	7,3	11,4	17,1	31,4	/	/	/
Displacement	$\delta_{V0}$	[mm]	1,9	2,4	4,0	4,3			
	$\delta_{V\infty}$	[mm]	2,9	3,6	5,9	6,4			

**Wedge Anchor BZ plus**

**Performance**  
Displacements under shear load

**Annex C9**

**Table C10:** Characteristic values for **tension loads, BZ-IG, cracked concrete**, static and quasi-static action, design method A according to ETAG 001, Annex C or CEN/TS 1992-4

Anchor size			M6	M8	M10	M12
Installation safety factor	$\gamma_2 = \gamma_{inst}$	[-]	1,2			
<b>Steel failure</b>						
Characteristic tension resistance, steel zinc plated	$N_{Rk,s}$	[kN]	16,1	22,6	26,0	56,6
Partial safety factor	$\gamma_{Ms}$	[-]	1,5			
Characteristic tension resistance, stainless steel A4, HCR	$N_{Rk,s}$	[kN]	14,1	25,6	35,8	59,0
Partial safety factor	$\gamma_{Ms}$	[-]	1,87			
<b>Pull-out failure</b>						
Characteristic resistance in cracked concrete C20/25	$N_{Rk,p}$	[kN]	5	9	12	20
Increasing factor	$\psi_c$	[-]	$\left(\frac{f_{ck,cube}}{25}\right)^{0,5}$			
<b>Concrete cone failure</b>						
Effective anchorage depth	$h_{ef}$	[mm]	45	58	65	80
Factor for cracked concrete	$k_{cr}$	[-]	7,2			

**Wedge Anchor BZ-IG**

**Performance**

Characteristic values for **tension loads, BZ-IG, cracked concrete**, static and quasi-static action, design method A according to ETAG 001, Annex C or CEN/TS 1992-4

**Annex C10**

**Table C11:** Characteristic values for **tension loads, BZ-IG, non-cracked concrete**, static and quasi-static action, design method A according to ETAG 001, Annex C or CEN/TS 1992-4

Anchor size			M6	M8	M10	M12
Installation safety factor	$\gamma_2 = \gamma_{inst}$	[-]	1,2			
<b>Steel failure</b>						
Characteristic tension resistance, steel zinc plated	$N_{Rk,s}$	[kN]	16,1	22,6	26,0	56,6
Partial safety factor	$\gamma_{Ms}$	[-]	1,5			
Characteristic tension resistance, stainless steel A4, HCR	$N_{Rk,s}$	[kN]	14,1	25,6	35,8	59,0
Partial safety factor	$\gamma_{Ms}$	[-]	1,87			
<b>Pull-out</b>						
Characteristic resistance in non-cracked concrete C20/25	$N_{Rk,p}$	[kN]	12	16	20	30
<b>Splitting</b> ( $N^0_{Rk,c}$ has to be replaced by $N^0_{Rk,sp}$ . The higher resistance of Case 1 and Case 2 may be applied.)						
Minimum thickness of concrete member	$h_{min}$	[mm]	100	120	130	160
<b>Case 1</b>						
Characteristic resistance in non-cracked concrete C20/25	$N^0_{Rk,sp}$	[kN]	9	12	16	25
Spacing (edge distance)	$s_{cr,sp} (= 2 c_{cr,sp})$	[mm]	3 $h_{ef}$			
<b>Case 2</b>						
Characteristic resistance in non-cracked concrete C20/25	$N^0_{Rk,sp}$	[kN]	12	16	20	30
Spacing (edge distance)	$s_{cr,sp} (= 2 c_{cr,sp})$	[mm]	5 $h_{ef}$			
Increasing factor for $N_{Rk,p}$ and $N^0_{Rk,sp}$	$\psi_c$	[-]	$\left(\frac{f_{ck,cube}}{25}\right)^{0,5}$			
<b>Concrete cone failure</b>						
Effective anchorage depth	$h_{ef}$	[mm]	45	58	65	80
Factor for non-cracked concrete	$k_{ucr}$	[-]	10,1			

**Wedge Anchor BZ-IG**

**Performance**

Characteristic values for **tension loads, BZ-IG, non-cracked concrete**, static and quasi-static action, design method A according to ETAG 001, Annex C or CEN/TS 1992-4

**Annex C11**

**Table C12:** Characteristic values for **shear loads, BZ-IG, cracked and non-cracked concrete**, static and quasi-static action, design method A according to ETAG 001, Annex C or CEN/TS 1992-4

Anchor size			M6	M8	M10	M12
Installation safety factor	$\gamma_2 = \gamma_{inst}$	[-]	1,0			
<b>BZ-IG, steel zinc plated</b>						
<b>Steel failure without lever arm, Installation type V</b>						
Characteristic shear resistance	$V_{Rk,s}$	[kN]	5,8	6,9	10,4	25,8
<b>Steel failure without lever arm, Installation type D</b>						
Characteristic shear resistance	$V_{Rk,s}$	[kN]	5,1	7,6	10,8	24,3
<b>Steel failure with lever arm, Installation type V</b>						
Characteristic bending resistance	$M^0_{Rk,s}$	[Nm]	12,2	30,0	59,8	104,6
<b>Steel failure with lever arm, Installation type D</b>						
Characteristic bending resistance	$M^0_{Rk,s}$	[Nm]	36,0	53,2	76,0	207
Partial safety factor for $V_{Rk,s}$ and $M^0_{Rk,s}$	$\gamma_{Ms}$	[-]	1,25			
Factor of ductility	$k_2$	[-]	1,0			
<b>BZ-IG, stainless steel A4, HCR</b>						
<b>Steel failure without lever arm, Installation type V</b>						
Characteristic shear resistance	$V_{Rk,s}$	[kN]	5,7	9,2	10,6	23,6
Partial safety factor	$\gamma_{Ms}$	[-]	1,25			
<b>Steel failure without lever arm, Installation type D</b>						
Characteristic shear resistance	$V_{Rk,s}$	[kN]	7,3	7,6	9,7	29,6
Partial safety factor	$\gamma_{Ms}$	[-]	1,25			
<b>Steel failure with lever arm, Installation type V</b>						
Characteristic bending resistance	$M^0_{Rk,s}$	[Nm]	10,7	26,2	52,3	91,6
Partial safety factor	$\gamma_{Ms}$	[-]	1,56			
<b>Steel failure with lever arm, Installation type D</b>						
Characteristic bending resistance	$M^0_{Rk,s}$	[Nm]	28,2	44,3	69,9	191,2
Partial safety factor	$\gamma_{Ms}$	[-]	1,25			
Factor of ductility	$k_2$	[-]	1,0			
<b>Concrete pry-out failure</b>						
k factor	$k_{(3)}$	[-]	1,5	1,5	2,0	2,0
<b>Concrete edge failure</b>						
Effective length of anchor in shear loading	$l_f$	[mm]	45	58	65	80
Effective diameter of anchor	$d_{nom}$	[mm]	8	10	12	16

**Wedge Anchor BZ-IG**

**Performance**

Characteristic values for **shear loads, BZ-IG, cracked and non-cracked concrete**, static and quasi-static action, design method A according to ETAG 001, Annex C or CEN/TS 1992-4

**Annex C12**

**Table C13:** Characteristic values for **tension** and **shear load** under **fire exposure**, **BZ-IG** cracked and non-cracked concrete C20/25 to C50/60, design acc. to TR 020 or CEN/TS 1992-4, Annex D

Anchor size		M6	M8	M10	M12		
<b>Tension load</b>							
<b>Steel failure</b>							
<b>Steel zinc plated</b>							
Characteristic resistance	R30	$N_{Rk,s,fi}$	[kN]	0,7	1,4	2,5	3,7
	R60			0,6	1,2	2,0	2,9
	R90			0,5	0,9	1,5	2,2
	R120			0,4	0,8	1,3	1,8
<b>Stainless steel A4, HCR</b>							
Characteristic resistance	R30	$N_{Rk,s,fi}$	[kN]	2,9	5,4	8,7	12,6
	R60			1,9	3,8	6,3	9,2
	R90			1,0	2,1	3,9	5,7
	R120			0,5	1,3	2,7	4,0
<b>Shear load</b>							
<b>Steel failure without lever arm</b>							
<b>Steel zinc plated</b>							
Characteristic resistance	R30	$V_{Rk,s,fi}$	[kN]	0,7	1,4	2,5	3,7
	R60			0,6	1,2	2,0	2,9
	R90			0,5	0,9	1,5	2,2
	R120			0,4	0,8	1,3	1,8
<b>Stainless steel A4, HCR</b>							
Characteristic resistance	R30	$V_{Rk,s,fi}$	[kN]	2,9	5,4	8,7	12,6
	R60			1,9	3,8	6,3	9,2
	R90			1,0	2,1	3,9	5,7
	R120			0,5	1,3	2,7	4,0
<b>Steel failure with lever arm</b>							
<b>Steel zinc plated</b>							
Characteristic resistance	R30	$M^0_{Rk,s,fi}$	[Nm]	0,5	1,4	3,3	5,7
	R60			0,4	1,2	2,6	4,6
	R90			0,4	0,9	2,0	3,4
	R120			0,3	0,8	1,6	2,8
<b>Stainless steel A4, HCR</b>							
Characteristic resistance	R30	$M^0_{Rk,s,fi}$	[Nm]	2,2	5,5	11,2	19,6
	R60			1,5	3,9	8,1	14,3
	R90			0,7	2,2	5,1	8,9
	R120			0,4	1,3	3,5	6,2

The characteristic resistance for pull-out failure, concrete cone failure, concrete pry-out failure and concrete edge failure can be designed according to TR020 / CEN/TS 1992-4.

**Wedge Anchor BZ-IG**

**Performance**

Characteristic values for **tension** and **shear loads** under **fire exposure**, **BZ-IG** cracked and non-cracked concrete C20/25 to C50/60, design acc. to TR 020 or CEN/TS 1992-4, Annex D

**Annex C13**

**Table C14: Displacements under tension load, BZ-IG**

Anchor size			M6	M8	M10	M12
Tension load in cracked concrete	N	[kN]	2,0	3,6	4,8	8,0
Displacements	$\delta_{N0}$	[mm]	0,6	0,6	0,8	1,0
	$\delta_{N\infty}$	[mm]	0,8	0,8	1,2	1,4
Tension load in non-cracked concrete	N	[kN]	4,8	6,4	8,0	12,0
Displacements	$\delta_{N0}$	[mm]	0,4	0,5	0,7	0,8
	$\delta_{N\infty}$	[mm]	0,8	0,8	1,2	1,4

**Table C15: Displacements under shear load, BZ-IG**

Anchor size			M6	M8	M10	M12
Shear load in cracked and non-cracked concrete	V	[kN]	4,2	5,3	6,2	16,9
Displacements	$\delta_{V0}$	[mm]	2,8	2,9	2,5	3,6
	$\delta_{V\infty}$	[mm]	4,2	4,4	3,8	5,3

**Wedge Anchor BZ-IG**

**Performance**  
Displacements under tension load and under shear load

**Annex C14**